

GEOTECHNICAL ENGINEERING

LAB MANUAL

Academic Year : 2017 - 2018
Course Code : ACE 105
Regulations : IARE – R16
Class : IV Semester
Branch : CIVIL ENGINEERING



Department of Civil Engineering

INSTITUTE OF AERONAUTICAL ENGINEERING

(Autonomous)

Dundigal, Hyderabad – 500 043



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Dundigal, Hyderabad - 500 043

DEPARTMENT OF CIVIL ENGINEERING

Program: Bachelor of Technology (B. Tech)

VISION OF THE DEPARTMENT

To produce eminent, competitive and dedicated civil engineers by imparting latest technical skills and ethical values to empower the students to play a key role in the planning and execution of infrastructural & developmental activities of the nation.

MISSION OF THE DEPARTMENT

To provide exceptional education in civil engineering through quality teaching, state-of-the-art facilities and dynamic guidance to produce civil engineering graduates, who are professionally excellent to face complex technical challenges with creativity, leadership, ethics and social consciousness.



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PROGRAM EDUCATIONAL OBJECTIVES (PEO's)

1. To impart proficiency in engineering knowledge and skills to analyze, design, build, maintain, or improve civil engineering based systems. **(Professional Excellence)**
2. To offer broad education and practical skills so that the students can carry out technical investigations within realistic constraints such as economic, environmental, societal, safety and sustainability. **(Understanding Socio-Economic Aspects)**
3. To impart ability to collaborate with and function on multidisciplinary teams to offer engineering solutions to the society **(Technical Collaboration)**
4. To create interest in the students to engage in life-long learning in advanced areas of civil engineering and related fields. **(Continued Self-Learning)**
5. To educate the students in ethical values and social responsibility to use engineering techniques and modern tools necessary for civil engineering practice to serve the society effectively. **(Effective Contribution to Society)**



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PROGRAM OUTCOMES (PO's)

PO1	Engineering knowledge: Apply the knowledge of mathematics, science, engineering fundamentals, and an engineering specialization to the solution of complex engineering problems.
PO2	Problem analysis: Identify, formulate, review research literature, and analyze complex engineering problems reaching substantiated conclusions using first principles of mathematics, natural sciences, and engineering sciences.
PO3	Design/development of solutions: Design solutions for complex engineering problems and design system components or processes that meet the specified needs with appropriate consideration for the public health and safety, and the cultural, societal, and environmental considerations.
PO4	Conduct investigations of complex problems: Use research-based knowledge and research methods including design of experiments, analysis and interpretation of data, and synthesis of the information to provide valid conclusions.
PO5	Modern tool usage: Create, select, and apply appropriate techniques, resources, and modern engineering and IT tools including prediction and modeling to complex engineering activities with an understanding of the limitations.
PO6	The engineer and society: Apply reasoning informed by the contextual knowledge to assess societal, health, safety, legal and cultural issues and the consequent responsibilities relevant to the professional engineering practice.
PO7	Environment and sustainability: Understand the impact of the professional engineering solutions in societal and environmental contexts, and demonstrate the knowledge of, and need for sustainable development.
PO8	Ethics: Apply ethical principles and commit to professional ethics and responsibilities and norms of the engineering practice.
PO9	Individual and team work: Function effectively as an individual, and as a member or leader in diverse teams, and in multidisciplinary settings.
PO10	Communication: Communicate effectively on complex engineering activities with the engineering community and with society at large, such as, being able to comprehend and write effective reports and design documentation, make effective presentations, and give and receive clear instructions.
PO11	Project management and finance: Demonstrate knowledge and understanding of the engineering and management principles and apply these to one's own work, as a member and leader in a team, to manage projects and in multidisciplinary environments.
PO12	Life-long learning: Recognize the need for, and have the preparation and ability to engage in independent and life-long learning in the broadest context of technological change.

PROGRAM SPECIFIC OUTCOMES (PSO's)

The Program Specific outcomes (PSO's) listed below were developed specifically to meet the Program Educational Objectives (PEO's). The focus of these PSO's is consistent with the set of required PO's identified in the NBA accreditation guidelines.

The Civil Engineering PSO's require that graduates receiving a Bachelor of Technology in Civil Engineering degree from IARE demonstrate the following.

PSO1. ENGINEERING KNOWLEDGE

Graduates shall demonstrate sound knowledge in analysis, design, laboratory investigations and construction aspects of civil engineering infrastructure, along with good foundation in mathematics, basic sciences and technical communication.

PSO2. BROADNESS AND DIVERSITY

Graduates will have a broad understanding of economical, environmental, societal, health and safety factors involved in infrastructural development, and shall demonstrate ability to function within multidisciplinary teams with competence in modern tool usage.

PSO3. SELF-LEARNING AND SERVICE

Graduates will be motivated for continuous self-learning in engineering practice and/or pursue research in advanced areas of civil engineering in order to offer engineering services to the society, ethically and responsibly.

MANDATORY INSTRUCTIONS

1. Students should report to the labs concerned as per the timetable.
2. Record should be updated from time to time and the previous experiment must be signed by the faculty in charge concerned before attending the lab.
3. Students who turn up late to the labs will in no case be permitted to perform the experiment scheduled for the day.
4. After completion of the experiment, certification of the staff in-charge concerned in the observation book is necessary.
5. Students should bring a notebook of about 100 pages and should enter the readings/observations/results into the notebook while performing the experiment.
6. The record of observations along with the detailed experimental procedure of the experiment performed in the immediate previous session should be submitted and certified by the staff member in-charge.
7. Not more than FIVE students in a group are permitted to perform the experiment on a set up.
8. The group-wise division made in the beginning should be adhered to, and no mix up of student among different groups will be permitted later.
9. The components required pertaining to the experiment should be collected from Lab- in-charge after duly filling in the requisition form.
10. When the experiment is completed, students should disconnect the setup made by them, and should return all the components/instruments taken for the purpose.

11. Any damage of the equipment or burnout of components will be viewed seriously either by putting penalty or by dismissing the total group of students from the lab for the semester/year.
12. Students should be present in the labs for the total scheduled duration.
13. Students are expected to prepare thoroughly to perform the experiment before coming to Laboratory.
14. Procedure sheets/data sheets provided to the students groups should be maintained neatly and are to be returned after the experiment.
15. DRESS CODE:
 1. Boys - Formal dress with tuck in and shoes.
 2. Girls - Formal dress (salwarkameez).
 3. Wearing of jeans is strictly prohibited

CONTENTS

S. No.		Page No
1	Determination of Moisture Content	09
2	Determination of Specific Gravity	11
3	Determination of Atterberg's Limits	14
4	Determination of Relative Density	23
5	Determination of Filed Density of Soil by a. Core Cutter method b. Sand Replacement method	28
6	Grain size distribution of the soil by Sieve Analysis	35
7	Determination of Co-efficient of Permeability of the Soil by a. Constant Head method b. Falling Head method	37
8	Determination of Optimum moisture content and maximum dry density of soil by Proctor Test	46
9	California Bearing Ration Test	49
10	Consolidation Test	54
11	Determination of Unconfined Compressive Strength of the Cohesive soil	68
12	Tri-axial compression Test	65
13	Determination of Shear Strength Parameters of soil by Direct Shear Test	70
14	Determination of Shear Strength Parameters of soil by Vane Shear Test	75
15	Standard Penetration Test	77

Experiment - 01

DETERMINATION OF MOISTURE CONTENT

Objective

Determine the natural moisture content of the given soil sample.

Need and Scope of the Experiment

In almost all soil tests natural moisture content of the soil is to be determined. The knowledge of the natural moisture content is essential in all studies of soil mechanics. To sight a few, natural moisture content is used in determining the bearing capacity and settlement. The natural moisture content will give an idea of the state of soil in the field.

Definition

The natural water content also called the natural moisture content is the ratio of the weight of water to the weight of the solids in a given mass of soil. This ratio is usually expressed as percentage.

Apparatus Required

1. Non-corrodible air-tight container.
2. Electric oven, maintain the temperature between 105°C to 110°C .
3. Desiccator.
4. Balance of sufficient sensitivity.

Procedure

1. Clean the containers with lid dry it and weigh it (W1).
2. Take a specimen of the sample in the container and weigh with lid (W2).
3. Keep the container in the oven with lid removed. Dry the specimen to constant weight maintaining the temperature between 105°C to 110°C for a period varying with the type of soil but usually 16 to 24 hours.
4. Record the final constant weight (W3) of the container with dried soil sample. Peat and other organic soils are to be dried at lower temperature (say 60°C) possibly for a longer period.

Certain soils contain gypsum which on heating loses its water if crystallization. If it is suspected that gypsum is present in the soil sample used for moisture content determination it shall be dried at not more than 80°C and possibly for a longer time.

Observations

Tab. 1.1 Data and observation sheet for water content determination

S. No.	Sample No.	1	2	3
1	Weight of container with lid (W_1 gm)			
2	Weight of container with lid +wet soil (W_2 gm)			
3	Weight of container with lid +dry soil (W_3 gm)			
4	Water/Moisture content $W = [(W_2 - W_3) / (W_3 - W_1)] * 100$			

Interpretation of Results

The natural moisture content of the given soil sample is _____ .

General Remarks

1. A container with out lid can be used, when moist sample is weighed immediately after placing the container and oven dried sample is weighed immediately after cooling in desiccator.
2. As dry soil absorbs moisture from wet soil, dried samples should be removed before placing wet samples in the oven.

Specimen Calculations

Experiment - 02

DETERMINATION OF SPECIFIC GRAVITY

Objective

Determine the specific gravity of the soil fraction passing 4.75 mm I.S sieve by density bottle

Need and scope of the experiment

The knowledge of specific gravity is needed in calculation of soil properties like void ratio, degree of saturation etc.

Definition

Specific gravity 'G' is defined as the ratio of the weight of an equal volume of distilled water at that temperature both weights taken in air.

Apparatus required

1. Density bottle of 50 ml with stopper having capillary hole.
2. Balance to weigh the materials (accuracy 10gm).
3. Wash bottle with distilled water.
4. Alcohol and ether.

Procedure

1. Clean and dry the density bottle
 - a. Wash the bottle with water and allow it to drain.
 - b. Wash it with alcohol and drain it to remove water.
 - c. Wash it with ether, to remove alcohol and drain ether.
2. Weigh the empty bottle with stopper (W_1)
3. Take about 10 to 20 gm of oven soil sample which is cooled in a desiccator. Transfer it to the bottle. Find the weight of the bottle and soil (W_2).
4. Put 10ml of distilled water in the bottle to allow the soil to soak completely. Leave it for about 2 hours.
5. Again fill the bottle completely with distilled water put the stopper and keep the bottle under constant temperature (T_x^0 C).
6. Take the bottle, wipe it clean and dry note. Now determine the weight of the bottle and the contents (W_3).
7. Now empty the bottle and thoroughly clean it. Fill the bottle with only distilled water and weigh it. Let it be W_4 at temperature (T_x^0 C).
8. Repeat the same process for 2 to 3 times, to take the average reading of it.

Observations

Table 2.1 Observations for the Specific gravity determination

S. No.	Observation Number	1	2	3
1	Weight of density bottle (W_1 g)			
2	Weight of density bottle + dry soil (W_2 g)			
3	Weight of bottle + dry soil + water at temperature T_x^0 C (W_3 g)			
4	Weight of bottle + water (W_4 g) at temperature T_x^0 C			
	Specific gravity G at T_x^0 C			
	Average specific gravity at T_x^0 C			

Calculations

$$\begin{aligned}
 \text{Specific gravity of soil} &= \frac{\text{Density of water at } 27^\circ \text{ C}}{\text{Weight of water of equal volume}} \\
 &= \frac{(W_2 - W_1)}{(W_4 - W_1) - (W_3 - W_2)} \\
 &= \frac{(W_2 - W_1)}{(W_2 - W_1) - (W_3 - W_4)}
 \end{aligned}$$

Interpretation

Unless or otherwise specified specific gravity values reported shall be based on water at 27°C . So the specific gravity at $27^\circ \text{C} = K * \text{Sp. gravity at } T_x^0 \text{C}$.

$$\text{where } K = \frac{\text{Density of water at temperature } T_x^0 \text{C}}{\text{Density of water at temperature } T_x^0 \text{C}}$$

Result

The specific gravity of the soil sample is _____ .

General Remarks

The specific gravity of the soil particles lie with in the range of 2.65 to 2.85. Soils containing organic matter and porous particles may have specific gravity values below 2.0. Soils having heavy substances may have values above 3.0.

Specimen Calculations

Experiment - 03

DETERMINATION OF ATTERBERG'S

Determination of Liquid Limit:

Objective

Determine the liquid limit for the given soil sample.

Need and scope

Liquid limit is significant to know the stress history and general properties of the soil met with construction. From the results of liquid limit the compression index may be estimated. The compression index value will help us in settlement analysis. If the natural moisture content of soil is closer to liquid limit, the soil can be considered as soft if the moisture content is lesser than liquids limit, the soil can be considered as soft if the moisture content is lesser than liquid limit. The soil is brittle and stiffer.

Theory

The liquid limit is the moisture content at which the groove, formed by a standard tool into the sample of soil taken in the standard cup, closes for 10 mm on being given 25 blows in a standard manner. At this limit the soil possess low shear strength.

Apparatus required

1. Balance
2. Liquid limit device (Casagrande's)
3. Grooving tool
4. Mixing dishes
5. Spatula
6. Electrical oven

Procedure

1. About 120 gm of air-dried soil from thoroughly mixed portion of material passing 425 micron I.S sieve is to be obtained.
2. Distilled water is mixed to the soil thus obtained in a mixing disc to form uniform paste. The paste shall have a consistency that would require 30 to 35 drops of cup to cause closer of standard groove for sufficient length.
3. A portion of the paste is placed in the cup of LIQUID LIMIT device and spread into portion with few strokes of spatula.

4. Trim it to a depth of 1cm at the point of maximum thickness and return excess of soil to the dish.
5. The soil in the cup shall be divided by the firm strokes of the grooving tool along the diameter through the centre line of the follower so that clean sharp groove of proper dimension is formed.
6. Lift and drop the cup by turning crank at the rate of two revolutions per second until the two halves of soil cake come in contact with each other for a length of about 1 cm by flow only.
7. The number of blows required to cause the groove close for about 1 cm shall be recorded.
8. A representative portion of soil is taken from the cup for water content determination.
9. Repeat the test with different moisture contents at least three more times for blows between 10 and 40.

Observations

Tab 7.1 Observation and Calculation sheet for the water content determination

Determination Number	1	2	3	4
Container number				
Weight of container				
Weight of container + wet soil				
Weight of container + dry soil				
Weight of water				
Weight of dry soil				
Moisture content (%)				
No. of blows				

Calculations

Draw a graph showing the relationship between water content (on y-axis) and number of blows (on x-axis) on semi-log graph. The curve obtained is called flow curve. The moisture content corresponding to 25 drops (blows) as read from the represents liquid limit. It is usually expressed to the nearest whole number.

Result

Liquid Limit of the given soil sample is _____

Flow index $I_f = (W_2 - W_1) / (\log N_1 / N_2) =$ slope of the flow curve.

Specimen Calculations

Determination of Plastic Limit:

Objective

Determine the plastic limit for the given soil sample.

Need and scope

Soil is used for making bricks, tiles and soil cement blocks in addition to its use as foundation for structures.

Apparatus required

1. Porcelain dish.
2. Glass plate for rolling the specimen.
3. Air tight containers to determine the moisture content.
4. Balance of capacity 200gm and sensitive to 0.01gm.
5. Oven thermostatically controlled with interior of non-corroding material to maintain the temperature around 105^o and 110^oc.

Procedure

1. Take about 20gm of thoroughly mixed portion of the material passing through 425 micron i.s. Sieve obtained in accordance with i.s. 2720 (part 1).
2. Mix it thoroughly with distilled water in the evaporating dish till the soil mass becomes plastic enough to be easily molded with fingers.
3. Allow it to season for sufficient time (for 24 hrs) to allow water to permeate throughout the soil mass.
4. Take about 10gms of this plastic soil mass and roll it between fingers and glass plate with just sufficient pressure to roll the mass into a threaded of uniform diameter throughout its length. The rate of rolling shall be between 60 and 90 strokes per minute.
5. Continue rolling till you get a threaded of 3 mm diameter.
6. Knead the soil together to a uniform mass and re-roll.
7. Continue the process until the thread crumbles when the diameter is 3 mm.
8. Collect the pieces of the crumbled thread in air tight container for moisture content determination.
9. Repeat the test to at least 3 times and take the average of the results calculated to the nearest whole number.

Observations

Compare the diameter of thread at intervals with the rod. When the diameter reduces to 3 mm, note the surface of the thread for cracks.

Container No.				
Wt. of container + lid, W_1				
Wt. of container + lid + wet sample, W_2				
Wt. of container + lid + dry sample, W_3				
Wt. of dry sample = $W_3 - W_1$				
Wt. of water in the soil = $W_3 - W_2$				
Water content (%) = $(W_3 - W_2) / (W_3 - W_1) * 100$				

Result

Average Plastic Limit for the given soil sample is _____

Specimen Calculations

Determination of Shrinkage Limit:

Objective

Determine the shrinkage limit and calculate the shrinkage ratio for the given soil sample.

Theory

As the soil loses moisture, either in its natural environment, or by artificial means in laboratory it changes from liquid state to plastic state, from plastic state to semi-solid state and then to solid state. Volume changes also occur with changes in water content. But there is particular limit at which any moisture change does not cause soil any volume change.

Need and scope

Soils which undergo large volume changes with change in water content may be troublesome. Usually volume changes may not be equal.

A shrinkage limit test should be performed on a soil.

1. To obtain a quantitative indication of how much change in moisture can occur before any appreciable volume changes occurs.
2. To obtain an indication of change in volume.
3. The shrinkage limit is useful in areas where soils undergo large volume changes when going through wet and dry cycles (as in case of earth dams)

Apparatus

1. Evaporating Dish. Porcelain, about 12cm diameter with flat bottom.
2. Spatula
3. Shrinkage Dish. Circular, porcelain or non-corroding metal dish (3 nos) having a flat bottom and 45 mm in diameter and 15 mm in height internally.
4. Straight Edge. Steel, 15 cm in length.
5. Glass cup. 50 to 55 mm in diameter and 25 mm in height, the top rim of which is ground smooth and level.
6. Glass plates. Two, each 75 , 75 mm one plate shall be of plain glass and the other shall have prongs.
7. Sieves. 2mm and 425- micron IS sieves.
8. Oven-thermostatically controlled.
9. Graduate-Glass, having a capacity of 25 ml and graduated to 0.2 ml and 100 cc one-mark flask.
10. Balance-Sensitive to 0.01 g minimum.
11. Mercury. Clean, sufficient to fill the glass cup to over flowing.
12. Wash bottle containing distilled water.

Procedure

Preparation of soil paste

1. Take about 100 gm of soil sample from a thoroughly mixed portion of the material passing through 425-micron I.S. sieve.
2. Place about 30 gm the above soil sample in the evaporating dish and thoroughly mixed with distilled water and make a creamy paste.
Use water content some where around the liquid limit.

Filling the shrinkage dish

3. Coat the inside of the shrinkage dish with a thin layer of Vaseline to prevent the soil sticking to the dish.
4. Fill the dish in three layers by placing approximately 1/3 rd of the amount of wet soil with the help of spatula. Tap the dish gently on a firm base until the soil flows over the edges and no apparent air bubbles exist. Repeat this process for 2nd and 3rd layers also till the dish is completely filled with the wet soil. Strike off the excess soil and make the top of the dish smooth. Wipe off all the soil adhering to the outside of the dish.
5. Weigh immediately, the dish with wet soil and record the weight.
6. Air- dry the wet soil cake for 6 to 8hrs, until the colour of the pat turns from dark to light. Then oven-dry the constant weight at 105⁰C to 110⁰C say about 12 to 16 hrs.
7. Remove the dried disk of the soil from oven. Cool it in a desiccator. Then obtain the weight of the dish with dry sample.
8. Determine the weight of the empty dish and record.
9. Determine the volume of shrinkage dish which is evidently equal to volume of the wet soil as follows. Place the shrinkage dish in an evaporating dish and fill the dish with mercury till it overflows slightly. Press it with plain glass plate firmly on its top to remove excess mercury. Pour the mercury from the shrinkage dish into a measuring jar and find the volume of the shrinkage dish directly. Record this volume as the volume of the wet soil pat.

Volume of the Dry Soil Pat

10. Determine the volume of dry soil pat by removing the pat from the shrinkage dish and immersing it in the glass cup full of mercury in the following manner.
11. Place the glass cup in a larger one and fill the glass cup to overflowing with mercury. Remove the excess mercury by covering the cup with glass plate with prongs and pressing it. See that no air bubbles are entrapped. Wipe out the outside of the glass cup to remove the adhering mercury. Then, place it in another larger dish, which is, clean and empty carefully.
12. Place the dry soil pat on the mercury. It floats submerge it with the pronged glass plate which is again made flush with top of the cup. The mercury spills over into the larger plate. Pour the mercury that is displaced by the soil pat into the measuring jar and find the volume of the soil pat directly.

Calculation

First determine the moisture content

$$\text{Shrinkage limit (WS)} = (W - (V - V_0) \times \gamma_w / W_0) \times 100$$

Where, W = Moisture content of wet soil pat (%)

V = Volume of wet soil pat in cm^3

V_0 = Volume of dry soil pat in cm^3

W_0 = Weight of oven dry soil pat in gm.

Caution

Do not touch the mercury with gold rings.

Tabulation and Results

Tab 9.1 Calculation sheet for shrinkage limit determination

S.No	Determination No.	1	2	3
1	Wt. of container in gm, W_1			
2	Wt. of container + wet soil pat in gm, W_2			
3	Wt. of container + dry soil pat in gm, W_3			
4	Wt. of oven dry soil pat, W_0 in gm			
5	Wt. of water in gm			
6	Moisture content (%), W			
7	Volume of wet soil pat (V), in cm^3			
8	Volume of dry soil pat (V_0) in cm^3			
9	By mercury displacement method			

	a. Weight of displaced mercury			
	b. Specific gravity of the mercury			
10	Shrinkage limit (W_s)			
11	Shrinkage ratio (R)			

Specimen Calculations

Experiment - 04

RELATIVE DENSITY TEST

Objective

To determine the relative density of given coarse grained material.

Planning and organization

Cushioned steel vibrating deck 75*75 cm size, R.P.M : 3600 ; under a 115 kg load, 440V, 3 phase supply.

Two cylindrical metallic moulds, 3000 cc and 15000 cc.

10 mm thick surcharge base plate with handle separately for each mould. Surcharge weights, one for each size having a weight equal to 140 gms / sq.cm.

Dial gauge holder, which can be slipped into the eyelets on the moulds sides.

Guide sleeves with clamps for each mould separately.

Calibration bar 75*300*3 mm.

Definitions

Relative density or density index is the ratio of the difference between the void ratios of a cohesion less soil in its loosest state and existing natural state to the difference between its void ratio in the loosest and densest states.

$$\text{Relative Density} = \frac{e_{\max} - e}{e_{\max} - e_{\min}}$$

Where,

e_{\max} = void ratio of coarse grained soil (cohesion less) in its loosest state.

e_{\min} = void ratio of coarse grained soil (cohesion less) in its densest state.

e = void ratio of coarse grained soil (cohesion less) in its natural existing state in the field.

Theory

Porosity of a soil depends on the shape of grain, uniformity of grain size and condition of sedimentation. Hence porosity itself does not indicate whether a soil is in loose or dense state. This information can only be obtained by comparing the porosity or void ratio of the given soil with that of the same soil in its loosest and densest possible state and hence the term, relative density is introduced.

$$\text{Relative Density} = \frac{e_{\max} - e}{e_{\max} - e_{\min}}$$

$$\text{We have, } e = V_v / V_s$$

$$\text{and } \gamma_d = G \gamma_w / (1 + e)$$

$$\therefore e = G \gamma_w / \gamma_d - 1$$

So, e is inversely proportional to its dry density (γ_d)

$$\text{Relative Density} = \frac{\frac{1}{(\gamma_d)_{\min}} - \frac{1}{(\gamma_d)}}{\frac{1}{(\gamma_d)_{\min}} - \frac{1}{(\gamma_d)_{\max}}}$$

Relative density is an arbitrary character of sandy deposit. In real sense, relative density expresses the ratio of actual decrease in volume of voids in a sandy soil to the maximum possible decrease in the volume of voids i.e how far the sand under investigation can be capable to the further densification beyond its natural state. Determination of relative density is helpful in compaction of coarse grained soils and in evaluating safe bearing capacity in case of sandy soils.

For very dense gravelly sand, it is possible to obtain relative density greater the one. This means that such natural dense packing could not be obtained in the laboratory.

Procedure

Calibration of mould:

1. Measure inside diameter of mould at different depths using a bore gauge and take the average.
2. Keep the mould on a flat surface or flat plate. Measure the height at different positions and take the average (accuracy = 0.025 mm).

3. Calculate the volume.
4. Fill the mould with distilled water till overflowing takes place.
5. Slide thick glass plate over the top surface of mould.
6. Weigh the water filling the mould.
7. Note the temperature of water.
8. Obtain density of water for the above temperature from physical tables.
9. Calculate the volume of the mould which is weight of water filling the mould /density of water.

Preparation of the Sample:

1. Dry the soil sample in a thermostatically controlled electric oven.
2. Cool in the sample in a desiccators.
3. Segregate soil lumps without breaking individual particles
4. Sieve it through the required sieve size.

Minimum Density:

The mould is weighed accurately (W). Pour the dry pulverized soil into the mould through a funnel in a steady stream. The spout is adjusted so that the free fall of soil particle is always 25 mm. While pouring soil the spout must have a spiral motion from the rim to the centre. The process is continued to fill up the mould with soil upto about 25mm above the top. It is then leveled, with the soil and weight is recorded (W₁).

Volume of mould V cc

Mass of dry soil M_s = (W₁ - W) gm

(γ_a)_{min} = M_s / V g / cc

e_{max} = Gγ_w / (γ_a)_{min} - 1

Maximum Density:

Weigh the empty mould (W). Put the collar on top of the mould and clamp it. Fill the mould with the oven dried soil sample till 1 / 2 or 2 / 3 of the collar is filled. Place the mould on the vibrating deck and fix it with nuts and bolts. Then place the surcharge weight on it. The vibrator is allowed to run for 8 minutes. Then mould is weighed with the soil and weight is recorded (W₂).

Volume of mould V cc

Mass of dry soil M_s = (W₂ - W) gm

(γ_d)_{max} = M_s / V g / cc

e_{min} = Gγ_w / (γ_d)_{max} - 1

Natural Density:

Weigh the mould with dry soil. Knowing the volume of the mould and weight of dry soil natural density, g_d, can be calculated.

$$e = G\gamma_w / (\gamma_d) - 1$$

$$\text{Relative Density} = \frac{e_{\max} - e}{e_{\max} - e_{\min}}$$

Calculation

1) Calculate the minimum index density (ρ_{dmin}) as follows:

$$\rho_{dmin} = M_{S1} / V_c$$

Where

M_{S1} = mass of tested-dry soil = Mass of mold with soil placed loose – mass of mold

V_c = Calibrated volume of the mold

(2) Calculate the maximum index density (ρ_{dmax}) as follows:

$$\rho_{dmax} = M_{S2} / V$$

Where

M_{S2} = mass of tested-dry soil = Mass of mold with soil after vibration – Mass of mold

V = Volume of tested-dry soil = V_c - (A_c*H)

Where

A_c = the calibrated cross sectional area of the mold

$$H = [R_f - R_i] + T_p$$

(3) Calculate the maximum and the minimum-index void ratios as follows (use G_s value as determined from specific gravity test; $\rho_w = 1 \text{ g/cm}^3$):

$$e_{\min} = [(\rho_w * G_s / \rho_{d\max}) - 1]$$

$$e_{\max} = [(\rho_w * G_s / \rho_{d\min}) - 1]$$

(4) Calculate the relative density as follows:

$$D_d = [(e_{\max} - e) / (e_{\max} - e_{\min})]$$

To calculate the void ratio (e) of the natural state of the soil, first calculate density of soil (ρ_d) and $\rho_s = G_s * \rho_w$ (use G_s value as determined from specific gravity test) as follows:

$$e = (\rho_s / \rho_d) - 1$$

Result:

The relative density of given coarse grained material =

Specimen Calculations

Experiment - 05

FIELD DENSITY TEST BY SAND REPLACEMENT METHOD

Objective

Determine the in situ density of natural or compacted soils using sand pouring cylinders.

Need and scope

The in-situ density of natural soil is needed for the determination of bearing capacity of soils, for the purpose of stability analysis of slopes, for the determination of pressures on underlying strata for the calculation of settlement and the design of underground structures.

It is very quality control test, where compaction is required, in the cases like embankment and pavement construction.

Apparatus required

1. Sand pouring cylinder of 3 litre/16.5 litre capacity mounted above a pouring cone and separated by a shutter cover plate.
2. Tools for excavating holes, suitable tools such as scraper tool to make a level surface.
3. Cylindrical calibrating container with an internal diameter of 100 mm/200 mm and an internal depth of 150 mm/250 mm fitted with a flange 50 mm/75 mm wide and about 5 mm surrounding the open end.
4. Balance to weigh upto an accuracy of 1g.
5. Metal containers to collect excavated soil.
6. Metal tray with 300 mm/450 mm square and 40 mm/50 mm deep with a 100 mm/200 mm diameter hole in the centre.
7. Glass plate about 450 mm/600 mm square and 10mm thick.
8. Clean, uniformly graded natural sand passing through 1.00 mm i.s.sieve and retained on the 600micron i.s.sieve. It shall be free from organic matter and shall have been oven dried and exposed to atmospheric humidity.
9. Suitable non-corrodible airtight containers.
10. Thermostatically controlled oven with interior on non-corroding material to maintain the temperature between 105⁰c to 110⁰c.
11. A desiccator with any desiccating agent other than sulphuric acid.

Theory

By conducting this test it is possible to determine the field density of the soil. The moisture content is likely to vary from time and hence the field density also. So it is required to report the test result in terms of dry density. The relationship that can be established between the dry density with known moisture content is as follows:

$$\gamma_d = \frac{\gamma_b}{(1 + w)}$$

$\gamma_d = \text{Dry density}$
 $\gamma_b = \text{Bulk density}$
 $w = \text{water content}$

Procedure

Calibration of the Cylinder

1. Fill the sand pouring cylinder with clean sand so that the level of the sand in the cylinder is within about 10 mm from the top. Find out the initial weight of the cylinder plus sand (W_1) and this weight should be maintained constant throughout the test for which the calibration is used.
2. Allow the sand of volume equal to that of the calibrating container to run out of the cylinder by opening the shutter, close the shutter and place the cylinder on the glass plate and open the shutter to allow the sand to run out and close the cylinder shutter when there is no movement of sand and remove the cylinder carefully. Weigh the sand collected on the glass plate. Its weight (W_2) gives the weight of sand filling the cone portion of the sand pouring cylinder.

Repeat this step at least three times and take the mean weight (W_2) Put the sand back into the sand pouring cylinder to have the same initial constant weight (W_1)

Determination of Bulk Density of Soil

3. Determine the volume (V) of the container by filling it with water to the brim. Check this volume by calculating from the measured internal dimensions of the container.
4. Place the sand pouring cylinder centrally on the calibrating container making sure that constant weight (W_1) is maintained. Open the shutter and permit the sand to run into the container. When no further movement of sand is seen close the shutter, remove the pouring cylinder and find its weight (W_3).

Determination of Dry Density of Soil In Place

5. Approximately 60 sqcm of area of soil to be tested should be trimmed down to a level surface, approximately of the size of the container. Keep the metal tray on the level surface and excavate a circular hole of volume equal to that of the calibrating container. Collect all the excavated soil in the

tray and find out the weight of the excavated soil (W_w). Remove the tray, and place the sand pouring cylinder filled to constant weight so that the base of the cylinder covers the hole concentrically. Open the shutter and permit the sand to run into the hole. Close the shutter when no further movement of the sand is seen. Remove the cylinder and determine its weight (W_3).

6. Keep a representative sample of the excavated sample of the soil for water content determination.

Observations and Calculations

Tab. 3.1 Calculation sheet for sand density determination

S.No.	Sample Details	1	2	3
	Calibration			
1	Weight of sand in cone (on glass plate from pouring cylinder) W_2 gm			
2	Volume of calibrating container (V) in cc			
3	Weight of sand + cylinder before pouring in calibrating container W_1 gm			
4	Weight of sand + cylinder after pouring in calibrating container W_3 gm			
5	Weight of sand filled in calibrating container $W_a = (W_1 - W_3 - W_2)$ gm			
6	Bulk density of sand $\rho_s = W_a / V$ gm/cc			

Tab. 3.2 Calculation sheet for density of soil

S. No.	Measurement of Soil Density	1	2	3
1	Weight of wet soil from hole W_w gm			
2	Weight of sand + cylinder before pouring into the hole & cone, W_1 gm			
3	Weight of sand + cylinder after pouring into the hole & cone, W_4 gm			
4	Weight of sand in hole $W_h = (W_1 - W_2 - W_4)$ gm			
5	Volume of the hole = $V_h = W_h / \rho_s$			
6	Bulk density $\rho_b = (W_w / V_h)$ gm/cc			
7	Water content determination			
8	Container number			
9	Weight of wet soil			
10	Weight of dry soil			
11	Moisture content (%)			
12	Dry density $\rho_d = \rho_b / (1+w)$ gm/cc			

General Remarks

1. Great care has to be taken while calibrating the bulk density of sand.
2. The excavated hole must be equal to the volume of the calibrating container.

Specimen Calculations

FIELD DENSITY TEST BY CORE CUTTER METHOD

Objective

Determine the dry density of the soil by using core cutter method.

Apparatus required

1. Cylindrical core cutter, 100 mm internal diameter and 130 mm long
2. Steel rammer, mass 9kg. Overall length, with the foot and staff about 900mm
3. Steel dolly, 25 mm high and 100mm internal diameter
4. Weighing balance, accuracy 1g.
5. Palette knife
6. Straight edge, steel rule etc

Theory

A cylindrical core cutter is a seamless steel tube. For determination of the dry density of the soil, the cutter is pressed into the soil mass so that it is filled with the soil. The cutter filled with the soil is lifted up. The mass of the soil is determined. The dry density is obtained as

$$\gamma_d = \frac{\gamma_b}{(1 + w)}$$

γ_d = Dry density

γ_b = Bulk density

w = water content

M = mass of wet soil in the cutter

V = internal volume of the cutter

Procedure

- 1) Determine the internal diameter and height of the core cutter to the nearest 0.25 mm.
- 2) Determine the mass (M_1) of the cutter to the nearest gram.
- 3) Expose a small area of the soil mass to be tested. Level the surface, about 300 mm square in area.
- 4) Place dolly over the top of the core cutter and press the core cutter in to the soil mass using the rammer. Stop the process of pressing when about 15 mm of the dolly protrudes above the soil surface.
- 5) Remove the soil surrounding the core cutter and takeout the core cutter. Some soil would project from the lower end of the cutter.
- 6) Remove the dolly. Trim the top and bottom surface of the core cutter carefully using a straight edge.

- 7) Weigh the core cutter filled with the soil to the nearest gram (M_2).
- 8) Remove the core of the soil from the cutter. Take a representative sample for the water content determination.

Observations and Calculations

Tab 4.1 Calculation sheet for dry density of soil

S.No	Observations	Sample No.		
		1	2	3
1	Internal diameter			
2	Internal Height			
3	Mass of empty core cutter (M_1)			
4	Mass of core cutter with soils (M_2)			
5	Mass of wet soil, $M = M_2 - M_1$			
6	Volume of the cutter, V			
7	Water content (%), w			
8	Bulk Density = M / V			
9	Dry density = Bulk Density / (1 + w)			

Result: The field dry density of the soil is _____.

Specimen Calculations

Experiment - 06

GRAIN SIZE DISTRIBUTION BY SIEVE ANALYSIS

Objective

Determine the relative proportions of different grain sizes which make up a given soil mass.

Need and scope of experiment

The grain size analysis is widely used in classification of soils. The data obtained from grain size distribution curves is used in the design of filters for earth dams and to determine suitability of soil for road construction, air field etc. Information obtained from grain size analysis can be used to predict soil water movement although permeability tests are more generally used.

Apparatus

1. Balance
2. I.s sieves
3. Rubber pestle and mortar
4. Manual/mechanical sieve shaker

Procedure

1. For soil samples of soil retained on 75 micron i.s sieve
 - (a) The proportion of soil sample retained on 75 micron i.s sieve is weighed and recorded weight of soil sample is as per i.s 2720.
 - (b) I.S.sieves are selected and arranged in the order as shown in the table.
 - (c) The soil sample is separated into various fractions by sieving through above sieves placed in the above mentioned order.
 - (d) The weight of soil retained on each sieve is recorded.
 - (e) The moisture content of soil if above 5% it is to be measured and recorded.
2. The sieves for soil tests: 4.75 mm to 75 microns.
3. No particle of soil sample shall be pushed through the sieves.
4. The balance to be used must be sensitive to the extent of 0.1% of total weight of sample taken.

Observations

Weight of soil sample taken:

Tab 5.1 Calculation sheet for soil particle % finer

S.No	I.S sieve number or size in mm	Wt. Retained in each sieve (gm)	Percentage on each sieve	Cumulative %age retained on each sieve	% finer	Remarks
1	4.75					
2	2.00					
3	1.00					
4	0.600					
5	0.425					
6	0.300					
7	0.150					
8	0.075					

Graph

Draw graph between log sieve size vs % finer. The graph is known as grading curve. Corresponding to 10%, 30% and 60% finer, obtain diameters from graph are designated as d_{10} , d_{30} , d_{60} .

Calculations

1. The percentage of soil retained on each sieve shall be calculated on the basis of total weight of soil sample taken.
2. Cumulative percentage of soil retained on each successive sieve is found and graph between log grain size of soil and % finer is drawn.

Specimen Calculations

Experiment - 7

PERMEABILITY TEST BY CONSTANT HEAD

Objective

Determine the coefficient of permeability of a given soil sample by using constant head method.

Need and scope

The knowledge of this property is much useful in solving problems involving yield of water bearing strata, seepage through earthen dams, stability of earthen dams, and embankments of canal bank affected by seepage, settlement etc.

Planning and organization

1. Preparation of the soil sample for the test
2. Finding the discharge through the specimen under a particular head of water.

Definition of coefficient of permeability

The rate of flow under laminar flow conditions through a unit cross sectional area of porous medium under unit hydraulic gradient is defined as coefficient of permeability.

Apparatus

1. Permeameter mould of non-corrodible material having a capacity of 1000 ml, with an internal diameter of 100mm and internal effective height of 127.3 mm.
2. The mould shall be fitted with a detachable base plate and removable extension counter.
3. Compacting equipment: 50 mm diameter circular face, weight 2.76 kg and height of fall 310 mm as specified in I.S 2720 part VII 1965.
4. Drainage bade: A bade with a porous disc, 12 mm thick which has the permeability 10 times the expected permeability of soil.
5. Drainage cap: A porous disc of 12 mm thick having a fitting for connection to water inlet or outlet.
6. Constant head tank: A suitable water reservoir capable of supplying water to the Permeameter under constant head.
7. Graduated glass cylinder to receive the discharge.
8. Stop watch to note the time.
9. A meter scale to measure the head differences and length of specimen.

Preparation of specimen for testing

A. Undisturbed soil sample

1. Note down the sample number, bore hole number and its depth at which the sample was taken.
2. Remove the protective cover (paraffin wax) from the sampling tube.
3. Place the sampling tube in the sample extraction frame, and push the plunger to get a cylindrical form sample not longer than 35 mm in diameter and having height equal to that of mould.
4. The specimen shall be placed centrally over the porous disc to the drainage base.
5. The angular space shall be filled with an impervious material such as cement slurry or wax, to provide sealing between the soil specimen and the mould against leakage from the sides.
6. The drainage cap shall then be fixed over the top of the mould.
7. Now the specimen is ready for the test.

B. Disturbed soil sample

1. A 2.5 kg sample shall be taken from a thoroughly mixed air dried or oven dried material.
2. The initial moisture content of the 2.5 kg sample shall be determined. Then the soil shall be placed in the air tight container.
3. Add required quantity of water to get the desired moisture content.
4. Mix the soil thoroughly.
5. Weigh the empty permeameter mould.
6. After greasing the inside slightly, clamp it between the compaction base plate and extension collar.
7. Place the assembly on a solid base and fill it with sample and compact it.
8. After completion of a compaction the collar and excess soil are removed.
9. Find the weight of mould with sample.
10. Place the mould with sample in the Permeameter, with drainage base and cap having discs that are properly saturated.

Test procedure

1. For the constant head arrangement, the specimen shall be connected through the top inlet to the constant head reservoir.
2. Open the bottom outlet.
3. Establish steady flow of water.
4. The quantity of flow for a convenient time interval may be collected.
5. Repeat three times for the same interval.

Observation and recording

The flow is very low at the beginning, gradually increases and then stands constant. Constant head permeability test is suitable for cohesion less soils. For cohesive soils falling head method is suitable.

Computation of result

Coefficient of permeability for a constant head test is given by

$$k = \frac{qL}{Ah}$$

where k = coefficient of permeability in cm/sec

q = Discharge cm^3/sec

L = Length of specimen in cm.

A = Cross-sectional area of specimen in cm^2

H = Constant head causing flow in cm.

Presentation of data

The coefficient of permeability is reported in cm/sec at 27° C. The dry density, the void ratio and the degree of saturation shall be reported. The test results should be tabulated as below:

Permeability Test Record

Project: _____ Tested By: _____ Location: _____
Boring No.: _____ Depth: _____

Details of sample

Diameter of specimen _____ cm

Length of specimen (L) _____ cm

Area of specimen (A) _____ cm^2

Specific gravity of soil G_s _____

Volume of specimen (V) _____ cm^3

Weight of dry specimen (W_s) _____ gm

Moisture content _____ %

Tab 10.1 Observation sheet for Permeability determination

Experiment No.		1	2	3
Length of specimen	L(cm)			
Area of specimen	A(cm ²)			
Time t	(sec)			
Discharge	q(cm ³)			
Height of water	h(cm)			
Temperature	(° C)			

Interpretation

1. Coefficient of Permeability $k_t = \frac{qL}{Ah}$

Discharge = Quantity of water collected (ml)/Time interval (sec)

2. The temperature correction shall be applied by the following formula :

$$k_{27} = k_t \times V_t / V_{27}$$

where k_{27} = coefficient of permeability at 27° C.

k_t = Coefficient of permeability at t° C.

V_t = Coefficient of viscosity at t° C.

V_{27} = Coefficient of viscosity at 27° C.

3. Void Ratio, $e = \frac{VG_s \gamma_w - W_s}{W_s}$

where V = Volume of specimen in cm³

G_s = Specific gravity of specimen

W_s = weight of dry specimen

γ_w = Density of water

γ_d = Dry density of soil sample

4. Degree of saturation

$$S_r = G_s w / e$$

Where w = Moisture content

e = Voids ratio.

Specimen Calculations

PERMEABILITY TEST BY FALLING HEAD METHOD

Objective

Determine the coefficient of permeability of the given soil sample, using falling head method.

Need and scope

The test results of the permeability experiments are used

1. To estimate ground water flow.
2. To calculate seepage through dams.
3. To find out the rate of consolidation and settlement of structures.
4. To plan the method of lowering the ground water table.
5. To calculate the uplift pressure and piping.
6. To design the grouting.
7. To design pits for recharging.
8. And also for soil freezing tests.

Thus the study of seepage of water through soil is very important, with wide field applications.

The falling head method of determining permeability is used for soil with low discharge, where as the constant head permeability test is used for coarse-grained soils with a reasonable discharge in a given time. For very fine-grained soil, capillarity permeability test is recommended.

Principle of the experiment

The passage of water through porous material is called seepage. A material with continuous voids is called a permeable material. Hence permeability is a property of a porous material which permits passage of fluids through inter connecting conditions. Hence permeability is defined as the rate of flow of water under laminar conditions through a unit cross-sectional area perpendicular to the direction of flow through a porous medium under unit hydraulic gradient and under standard temperature conditions.

The principle behind the test is Darcy's law for laminar flow. The rate of discharge is proportional to $(i \times A)$
 $q = kiA$

Where, q = Discharge per unit time.

A = Total area of c/s of soil perpendicular to the direction of flow.

i = hydraulic gradient.

k = Darcy's coefficient of permeability

= the mean velocity of flow that will occur through the cross-sectional area under unit hydraulic gradient.

Apparatus

1. Permeameter with its accessories.
2. Standard soil specimen.
3. Deaired water.
4. Balance to weigh up to 1 gm.
5. I.s sieves 4.75 mm and 2 mm.
6. Mixing pan.
7. Stop watch.
8. Measuring jar.
9. Meter scale.
10. Thermometer.
11. Container for water.
12. Trimming knife etc.

Knowledge of equipment

- (a) The permeameter is made of non-corrodible material with a capacity of 1000 ml, with an internal diameter of 100/0.1 mm and effective height of 127.3/ 0.1 mm.
- (b) The mould has a detachable base plate and a removable exterior collar.
- (c) The compacting equipment has a circular face with 50 mm diameter and a length of 310 mm with a weight of 2.6 kg.
- (d) The drainage base is a porous disc, 12 mm thick with a permeability 10 times that of soil.
- (e) The drainage cap is also a porous disc of 12 mm thickness with an inlet/outlet fitting.
- (f) The container tank has an overflow valve. There is also a graduated jar to collect discharge.
- (g) The stand pipe arrangements are done on a board with 2 or 3 glass pipes of different diameters.

Preparation of the specimen

The preparation of the specimen for this test is important. There are two types of specimen, the undisturbed soil sample and the disturbed or made up soil sample.

A. Undisturbed soil specimen

The preparation of the sample is as follows.

1. Note down-sample no., borehole no., depth at which sample is taken.
2. Remove the protective cover (wax) from the sampling tube.
3. Place the sampling tube in the sample extractor or and push the plunger to get a cylindrical shaped specimen not larger than 85 mm diameter and height equal to that of the mould.
4. This specimen is placed centrally over the drainage disc of base plate.
5. The annular space in between the mould and specimen is filled with an impervious material like cement slurry to block the side leakage of the specimen.
6. Protect the porous disc when cement slurry is poured.

7. Compact the slurry with a small tamper.
8. The drainage cap is also fixed over the top of the mould.
9. The specimen is now ready for test.

B. Disturbed specimen

The disturbed specimen can be prepared by static compaction or by dynamic compaction.

(a) Preparation of statically compacted (disturbed) specimen

1. Take 800 to 1000 gms of representative soil and mix with water to O.M.C determined by I.S Light Compaction test. Then leave the mix for 24 hours in an airtight container.
2. Find weight W of soil mix for the given volume of the mould and hence find the dry density γ_d for $W = \gamma_d (1 + W) V$ by weighing correct to 1 gm.
3. Now, assemble the permeameter for static compaction. Attach the 3 cm collar to the bottom end of 0.3 liters mould and the 2 cm collar to the top end. Support the mould assembly over 2.5 cm end plug, with 2.5 cm collar resting on the split collar kept around the 2.5 cm- end plug. The inside of the 0.3 lit. Mould is lightly greased.
4. Put the weighed soil into the mould. Insert the top 3 cm end plug into the top collar, tamping the soil with hand.
5. Keep, now the entire assembly on a compressive machine and remove the split collar. Apply the compressive force till the flanges of both end plugs touch the corresponding collars. Maintain this load for 1 mt and then release it.
6. Then remove the top 3 cm plug and collar place a filter paper on fine wire mesh on the top of the specimen and fix the perforated base plate.
7. Turn the mould assembly upside down and remove the 2.5 cm end plug and collar. Place the top perforated plate on the top of the soil specimen and fix the top cap on it, after inserting the seating gasket.
8. Now the specimen is ready for test.

(b) Preparation of Dynamically Compacted Disturbed sample

1. Take 800 to 1000 gm of representative soil and mix it with water to get O.M.C, if necessary. Have the mix in airtight container for 24 hours.
2. Assemble the permeameter for dynamic compaction. Grease the inside of the mould and place it upside down on the dynamic compaction base. Weigh the assembly correct to a gram (w). Put the 3 cm collar to the other end.
3. Now, compact the wet soil in 2 layers with 15 blows to each layer with a 2.5 kg dynamic tool. Remove the collar and then trim off the excess. Weigh the mould assembly with the soil (W_2).
4. Place the filter paper or fine wore mesh on the top of the soil specimen and fix the perforated base plate on it.
5. Turn the assembly upside down and remove the compaction plate. Insert the sealing gasket and place the top perforated plate on the top of soil specimen. And fix the top cap.
6. Now, the specimen is ready for test.

Experimental Procedure

1. Prepare the soil specimen as specified.
2. Saturate it. Deaired water is preferred.
3. Assemble the permeameter in the bottom tank and fill the tank with water.
4. Inlet nozzle of the mould is connected to the stand pipe. Allow some water to flow until steady flow is obtained.
5. Note down the time interval t for a fall of head in the stand pipe h .
6. Repeat step 5 three times to determine t for the same head.
7. Find a by collecting q for the stand pipe. Weigh it correct to 1 gm and find a from $q/h=a$.

Therefore the coefficient of permeability

$$k = \frac{2.3 \times a \times L \times (\log_{10} h_{21} / h_2)}{A \times t} \quad \text{cm/sec}$$

K at standard temperature of $27^\circ \text{C} = K \times \gamma_t / \gamma_{27}$

γ_t = Viscosity of water at temperature $t^\circ \text{C}$

γ_{27} = Viscosity of water at room temperature 27°C

Interpretation of the result

There are high values, medium values and low values for permeability

$K > 10^{-1} \text{ cm/sec}$, the permeability is high

$= 10^{-1} \text{ cm/sec}$, it is medium

$< 10^{-1} \text{ cm/sec}$, it is low.

Observations

Sample No.

Moulding water content :

Dry density :

$\mu_{27} =$

Specific gravity :

$\gamma_{27} =$

voids ratio :

Tab 11.1 Observation sheet for coefficient of permeability determination

S.No	Details	1 st set	2 nd set
1	Area of stand pipe (dia. 5 cm) a		
2	Cross sectional area of soil specimen A		
3	Length of soil specimen L		
4	Initial reading of stand pipe h_1		
5	Final reading of stand pipe h_2		
6	Time t		
7	Test temperature T		
8	Coefficient of permeability at T k_t		
9	Coefficient of permeability at 27° C k_{27}		

General Remarks

1. During test there should be no volume change in the soil, there should be no compressible air present in the voids of soil i.e. soil should be completely saturated. The flow should be laminar and in a steady state condition.
2. Coefficient of permeability is used to assess drainage characteristics of soil, to predict rate of settlement founded on soil bed.

Specimen Calculations

Experiment - 08

STANDARD & MODIFIED PROCTOR TEST

Objective

Determination of the relationship between the moisture content and density of soils compacted in a mould of a given size.

Apparatus

1. Proctor mould having a capacity of 944 cc with an internal diameter of 100mm and effective height of 127.3mm for standard proctor test and 2250 cc with an internal diameter of 150mm and effective height of 127.3mm for modified proctor test. The mould shall have a detachable collar assembly and a detachable base plate.
2. Rammer: a mechanical operated metal rammer of weight of 2.6 kg, drop of 310mm and weight of 4.9 kg, drop of 450mm for standard and modified proctor test respectively. The rammer shall be equipped with a suitable arrangement to control the height of drop to a free fall.
3. Sample extruder.
4. A balance of 15 kg capacity.
5. Sensitive balance.
6. Straight edge.
7. Graduated cylinder.
8. Mixing tools such as mixing pan, spoon, towel, spatula etc.
9. Moisture tins.

Procedure

1. Take a representative oven-dried sample, approximately 3/5 kg in the given pan. Thoroughly mix the sample with sufficient water to dampen it to approximately four to six percentage points below optimum moisture content.
2. Weigh the proctor mould without base plate and collar. Fix the collar and base plate. Place the soil in the Proctor mould and compact it in 3 layers giving 25 blows per layer with the 2.6 kg rammer falling through and in 5 layers giving 56 blows per layer with the 4.9 kg rammer falling through for standard and modified test respectively.
3. Remove the collar, trim the compacted soil even with the top of the mould by means of the straight edge and weigh.
4. Divide the weight of the compacted specimen and record the result as the wet weight γ_{wet} in grams per cubic centimeter of the compacted soil.
5. Remove the sample from the mould and slice vertically through and obtain a small sample for moisture determination.
6. Thoroughly break up the remainder of the material and add water in sufficient amounts to increase the moisture content of the soil sample by one or two percentage points and repeat the above procedure for each increment of water added.
7. Continue this series of determination until there is either a decrease or no change in the wet unit weight of the compacted soil.

Calculation

Wet density gm/cc = weight of compacted soil / 944. (2250 for modified)

Dry density = wet density / (1+w)

Where w is the moisture content of the soil.

Plot the dry density against moisture content and find out the maximum dry density and optimum moisture for the soil.

Observations

Cylinder diameter _____ cm Height _____ cm Volume _____ cc

Weight of empty cylinder _____ gm

Tab 12.1 Calculation sheet for soil dry density determination

Details	1	2	3	4	5
Water to be added (percent)					
Weight of water to be added (gm)					
Weight of cylinder + compacted soil					
Weight of compacted soil (gms)					
Container No.					
Wt. Of container + wet soil in gms					
Wt. Of container + dry soil in gms					
Wt of empty container in gms.					
Water content in percentage					
Average moisture content (percent)					
Wet density (gm /cc)					
Dry density (gm/cc)					

Result

The Maximum Dry Density and Optimum Moisture Content are _____ & _____ respectively.

Specimen Calculations

Experiment - 09

CALIFORNIA BEARING RATIO TEST

Objective

Determine the california bearing ratio by conducting a load penetration test in the laboratory.

Need and scope

The california bearing ratio test is penetration test meant for the evaluation of sub grade strength of roads and pavements. The results obtained by these tests are used with the empirical curves to determine the thickness of pavement and its component layers. This is the most widely used method for the design of flexible pavement.

This instruction sheet covers the laboratory method for the determination of c.b.r. Of undisturbed and remoulded /compacted soil specimens, both in soaked as well as unsoaked state.

Planning and organization

Equipments and tool required are

1. Cylindrical mould with inside dia 150 mm and height 175 mm, provided with a detachable extension collar 50 mm height and a detachable perforated base plate 10 mm thick.
2. Spacer disc 148 mm in dia and 47.7 mm in height along with handle.
3. Metal rammers. Weight 2.6 kg with a drop of 310 mm (or) weight 4.89 kg a drop 450 mm.
4. Weights. One annular metal weight and several slotted weights weighing 2.5 kg each, 147 mm in dia, with a central hole 53 mm in diameter.
5. Loading machine. With a capacity of at least 5000 kg and equipped with a movable head or base that travels at an uniform rate of 1.25 mm/min. Complete with load indicating device.
6. Metal penetration piston 50 mm dia and minimum of 100 mm in length.
7. Two dial gauges reading to 0.01 mm.
8. Sieves. 4.75 mm and 20 mm i.s. Sieves.
9. Miscellaneous apparatus, such as a mixing bowl, straight edge, scales soaking tank or pan, drying oven, filter paper and containers.

Definition of CBR

It is the ratio of force per unit area required to penetrate a soil mass with standard circular piston at the rate of 1.25 mm/min. to that required for the corresponding penetration of a standard material.

$$\text{C.B.R.} = \text{Test load/Standard load} * 100$$

The following table gives the standard loads adopted for different penetrations for the standard material with a C.B.R. value of 100%

Tab 18.1 values of Penetration vs Standard load

Penetration of plunger (mm)	Standard load (kg)
2.5	1370
5	2055
7.5	2630
10	3180
12.5	3600

The test may be performed on undisturbed specimens and on remoulded specimens which may be compacted either statically or dynamically.

Preparation of Test Specimen

(A) Undisturbed specimen

Attach the cutting edge to the mould and push it gently into the ground. Remove the soil from the outside of the mould which is pushed in. When the mould is full of soil, remove it from weighing the soil with the mould or by any field method near the spot.

Determine the density

(B) Remoulded specimen

Prepare the remoulded specimen at Proctor's maximum dry density or any other density at which C.B.R. is required. Maintain the specimen at optimum moisture content or the field moisture as required. The material used should pass 20 mm I.S. sieve but it should be retained on 4.75 mm I.S. sieve. Prepare the specimen either by dynamic compaction or by static compaction.

Dynamic Compaction

1. Take about 4.5 to 5.5 kg of soil and mix thoroughly with the required water.

2. Fix the extension collar and the base plate to the mould. Insert the spacer disc over the base (See Fig.38). Place the filter paper on the top of the spacer disc.
3. Compact the mix soil in the mould using either light compaction or heavy compaction. For light compaction, compact the soil in 3 equal layers, each layer being given 55 blows by the 2.6 kg rammer. For heavy compaction compact the soil in 5 layers, 56 blows to each layer by the 4.89 kg rammer.
4. Remove the collar and trim off soil.
5. Turn the mould upside down and remove the base plate and the displacer disc.
6. Weigh the mould with compacted soil and determine the bulk density and dry density.
7. Put filter paper on the top of the compacted soil (collar side) and clamp the perforated base plate on to it.

Static compaction

1. Calculate the weight of the wet soil at the required water content to give the desired density when occupying the standard specimen volume in the mould from the expression.

$$W = \text{desired dry density} * (1+w) V$$

2. Where W = Weight of the wet soil
3. w = desired water content
4. V = volume of the specimen in the mould = 2250 cm³ (as per the mould available in laboratory)
5. Take the weight W (calculated as above) of the mix soil and place it in the mould.
6. Place a filter paper and the displacer disc on the top of soil.
7. Keep the mould assembly in static loading frame and compact by pressing the displacer disc till the level of disc reaches the top of the mould.
8. Keep the load for some time and then release the load. Remove the displacer disc.
9. The test may be conducted for both soaked as well as unsoaked conditions.
10. If the sample is to be soaked, in both cases of compaction, put a filter paper on the top of the soil and place the adjustable stem and perforated plate on the top of filter paper.
11. Put annular weights to produce a surcharge equal to weight of base material and pavement expected in actual construction. Each 2.5 kg weight is equivalent to 7 cm construction. A minimum of two weights should be put.
12. Immerse the mould assembly and weights in a tank of water and soak it for 96 hours. Remove the mould from tank.
13. Note the consolidation of the specimen.

Procedure for Penetration Test

1. Place the mould assembly with the surcharge weights on the penetration test machine.
2. Seat the penetration piston at the center of the specimen with the smallest possible load, but in no case in excess of 4 kg so that full contact of the piston on the sample is established.
3. Set the stress and strain dial gauge to read zero. Apply the load on the piston so that the penetration rate is about 1.25 mm/min.

If the initial portion of the curve is concave upwards, apply correction by drawing a tangent to the curve at the point of greatest slope and shift the origin. Find and record the correct load reading corresponding to each penetration.

$$\text{C.B.R.} = P_T/P_S * 100$$

Where P_T = Corrected test load corresponding to the chosen penetration from the load penetration curve.

P_S = Standard load for the same penetration taken from the table I.

Tab 18.2 Tabulation for CBR test

Penetration Dial		Load Dial		Corrected Load
Readings	Penetration (mm)	Proving ring reading	Load (kg)	

Interpretation and recording

C.B.R. of specimen at 2.5 mm penetration:

C.B.R. of specimen at 5.0 mm penetration:

General Remarks

The C.B.R. values are usually calculated for penetration of 2.5 mm and 5 mm. Generally the C.B.R. value at 2.5 mm will be greater than value at 5 mm and in such a case/the former shall be taken as C.B.R. for design purpose. If C.B.R. for 5 mm exceeds that for 2.5 mm, the test should be repeated. If identical results follow, the C.B.R. corresponding to 5 mm penetration should be taken for design

Specimen Calculations

Experiment - 10

CONSOLIDATION TEST

Objective

Determine the settlements due to primary consolidation of soil by conducting one dimensional test.

Need and scope

The test is conducted to determine the settlement due to primary consolidation. To determine the following

1. Rate of consolidation under normal load.
2. Degree of consolidation at any time.
3. Pressure-void ratio relationship.
4. Coefficient of consolidation at various pressures.
5. Compression index.

From the above information it will be possible for us to predict the time rate and extent of settlement of structures founded on fine-grained soils. It is also helpful in analyzing the stress history of soil. Since the settlement analysis of the foundation depends mainly on the values determined by the test, this test is very important for foundation design.

Planning and organization

1. Consolidometer consisting essentially
 - a) A ring of diameter = 60mm and height = 20mm
 - b) Two porous plates or stones of silicon carbide, aluminum oxide or porous metal.
 - c) Guide ring.
 - d) Outer ring.
 - e) Water jacket with base.
 - f) Pressure pad.
 - g) Rubber basket.
2. Loading device consisting of frame, lever system, loading yoke dial gauge fixing device and weights.
3. Dial gauge to read to an accuracy of 0.002mm.
4. Thermostatically controlled oven.
5. Stopwatch to read seconds.

6. Sample extractor.
7. Miscellaneous items like balance, soil trimming tools, spatula, filter papers, sample containers.

Principle

When a compressive load is applied to soil mass, a decrease in its volume takes place, the decrease in volume of soil mass under stress is known as compression and the property of soil mass pertaining to its tendency to decrease in volume under pressure is known as compressibility. In a saturated soil mass having its void filled with incompressible water, decrease in volume or compression can take place when water is expelled out of the voids. Such a compression resulting from a long time static load and the consequent escape of pore water is termed as consolidation.

Then the load is applied on the saturated soil mass, the entire load is carried by pore water in the beginning. As the water starts escaping from the voids, the hydrostatic pressure in water gets gradually dissipated and the load is shifted to the soil solids which increases effective on them, as a result the soil mass decrease in volume. The rate of escape of water depends on the permeability of the soil.

- 1) From the sample tube, eject the sample into the consolidation ring. The sample should project about one cm from outer ring. Trim the sample smooth and flush with top and bottom of the ring by using a knife. Clean the ring from outside and keep it ready from weighing.
- 2) Remolded sample:
 - a) Choose the density and water content at which sample has to be compacted from the moisture density relationship.
 - b) Calculate the quantity of soil and water required to mix and compact.
 - c) Compact the specimen in compaction mould in three layers using the standard rammers.
 - d) Eject the specimen from the mould using the sample extractor.

Procedure

1. Saturate two porous stones either by boiling in distilled water about 15 minute or by keeping them submerged in the distilled water for 4 to 8 hrs. Wipe away excess water. Fittings of the consolidometer which is to be enclosed shall be moistened.
2. Assemble the consolidometer, with the soil specimen and porous stones at top and bottom of specimen, providing a filter paper between the soil specimen and porous stone. Position the pressure pad centrally on the top porous stone.
3. Mount the mould assembly on the loading frame, and center it such that the load applied is axial.
4. Position the dial gauge to measure the vertical compression of the specimen. The dial gauge holder should be set so that the dial gauge is in the begging of its releases run, allowing sufficient margin for the swelling of the soil, if any.

5. Connect the mould assembly to the water reservoir and the sample is allowed to saturate. The level of the water in the reservoir should be at about the same level as the soil specimen.
6. Apply an initial load to the assembly. The magnitude of this load should be chosen by trial, such that there is no swelling. It should be not less than 50 g/cm^3 for ordinary soils & 25 g/cm^2 for very soft soils. The load should be allowed to stand until there is no change in dial gauge readings for two consecutive hours or for a maximum of 24 hours.
7. Note the final dial reading under the initial load. Apply first load of intensity 0.1 kg/cm^2 start the stop watch simultaneously. Record the dial gauge readings at various time intervals. The dial gauge readings are taken until 90% consolidation is reached. Primary consolidation is gradually reached within 24 hrs.
8. At the end of the period, specified above take the dial reading and time reading. Double the load intensity and take the dial readings at various time intervals. Repeat this procedure fir successive load increments. The usual loading intensity are as follows :
 - a. 0.1, 0.2, 0.5, 1, 2, 4 and 8 kg/cm^2 .
9. After the last loading is completed, reduce the load to the value of the last load and allow it to stand for 24 hrs. Reduce the load further in steps of the previous intensity till an intensity of 0.1 kg/cm^2 is reached. Take the final reading of the dial gauge.
10. Reduce the load to the initial load, keep it for 24 hrs and note the final readings of the dial gauge.
11. Quickly dismantle the specimen assembly and remove the excess water on the soil specimen in oven, note the dry weight of it.

Observation and Reading

Data and observation sheet for consolidation test pressure, compression and time.

Name of the project

Borehole no:

Depth of the sample:

Description of soil:

Empty weight of ring:

Area of ring:

Diameter of ring:

Volume of ring:

Height of ring:

Specific gravity of soil sample:

Dial Gauge (least count)

Tab 17.1 Observation sheet for time- pressure of consolidation test

Pressure Intensity (Kg/cm²)	0.1	0.2	0.5	1	2	4	8
Elapsed Time							
0.25							
1							
2.5							
4							
6.25							
9							
16							
25							
30							
1 hr							
2 hrs							
4 hrs							
8 hrs							
24 hrs							

Tab 17.2 Observation Sheet for Consolidation Test: Pressure Voids Ratio

Applied pressure	Final dial reading	Dial change	Specimen height	Height of solids	Height of voids	Void ratio
0						
0.1						
0.2						
0.5						
1						
2						
4						
8						
4						
2						
1						
0.5						
0.2						
0.1						

Calculations

1. Height of solids (H_S) is calculated from the equation

$$H_S = W_S/G * A$$

2. Void ratio. Voids ratio at the end of various pressures are calculated from equation

$$e = (H - H_S)/H_S$$

3. Coefficient of consolidation. The Coefficient of consolidation at each pressures increment is calculated by using the following equations:

- i. $C_v = 0.197 d^2/t_{50}$ (Log fitting method)
- ii. $C_v = 0.848 d^2/t_{90}$ (Square fitting method)

In the log fitting method, a plot is made between dial readings and logarithmic of time, the time corresponding to 50% consolidation is determined.

In the square root fitting method, a plot is made between dial readings and square root of time and the time corresponding to 90% consolidation is determined. The values of C_v are recorded in table II.

4. Compression Index. To determine the compression index, a plot of voids ratio (e) V_s log t is made. The initial compression curve would be a straight line and the slope of this line would give the compression index C_c .

5. Coefficient of compressibility. It is calculated as follows

$$a_v = 0.435 C_c / \text{Avg. pressure for the increment}$$

where C_c = Coefficient of compressibility

6. Coefficient of permeability. It is calculated as follows

$$K = C_v \cdot a_v \cdot (\text{unit weight of water}) / (1+e).$$

Graphs

1. Dial reading V_s log of time or
Dial reading V_s square root of time.
2. Voids ratio V_s log σ (average pressure for the increment).

General Remarks

1. While preparing the specimen, attempts has to be made to have the soil strata orientated in the same direction in the consolidation apparatus.
2. During trimming care should be taken in handling the soil specimen with least pressure.
3. Smaller increments of sequential loading have to be adopted for soft soils.

Specimen Calculations

Experiment - 11

UNCONFINED COMPRESSION TEST

Objective

To determine shear parameters of cohesive soil

Need and scope

It is not always possible to conduct the bearing capacity test in the field. Some times it is cheaper to take the undisturbed soil sample and test its strength in the laboratory. Also to choose the best material for the embankment, one has to conduct strength tests on the samples selected. Under these conditions it is easy to perform the unconfined compression test on undisturbed and remoulded soil sample. Investigate experimentally the strength of a given soil sample.

Planning and organization

Find out the diameter and length of the specimen.

Equipment

1. Loading frame of capacity of 2 t, with constant rate of movement. (note the least count of the dial gauge attached to the proving ring)
2. Proving ring of 0.01 kg sensitivity for soft soils; 0.05 kg for stiff soils
3. Soil trimmer
4. Frictionless end plates of 75 mm diameter (perspex plate with silicon grease coating)
5. Evaporating dish (aluminum container)
6. Soil sample of 75 mm length
7. Dial gauge (0.01 mm accuracy)
8. Balance of capacity 200 g and sensitivity to weigh 0.01 g
9. Oven thermostatically controlled with interior of non-corroding material to maintain the temperature at the desired level
10. Sample extractor and split sampler
11. Dial gauge (sensitivity 0.01mm)
12. Vernier calipers
- 13.

Experimental procedure (specimen)

In this test, a cylinder of soil without lateral support is tested to failure in simple compression, at a constant rate of strain. The compressive load per unit area required to fail the specimen as called unconfined compressive strength of the soil.

Preparation of specimen for testing

(A) Undisturbed specimen

1. Note down the sample number, bore hole number and the depth at which the sample was taken.
2. Remove the protective cover (paraffin wax) from the sampling tube.
1. Place the sampling tube extractor and push the plunger till a small length of sample moves out.
2. Trim the projected sample using a wire saw.
3. Again push the plunger of the extractor till a 75 mm long sample comes out.
4. Cutout this sample carefully and hold it on the split sampler so that it does not fall.
5. Take about 10 to 15 g of soil from the tube for water content determination.
6. Note the container number and take the net weight of the sample and the container.
7. Measure the diameter at the top, middle, and the bottom of the sample and find the average and record the same.
8. Measure the length of the sample and record.
9. Find the weight of the sample and record.

(B) Disturbed sample

1. For the desired water content and the dry density, calculate the weight of the dry soil W_s required for preparing a specimen of 3.8 cm diameter and 7.5 cm long.
2. Add required quantity of water W_w to this soil. $W_w = W_s - W/100$ gm
3. Mix the soil thoroughly with water.
4. Place the wet soil in a tight thick polythene bag in a humidity chamber and place the soil in a constant volume mould, having an internal height of 7.5 cm and internal diameter of 3.8 cm.
5. After 24 hours take the soil from the humidity chamber and place the soil in a constant volume mould, having an internal height of 7.5 cm and internal diameter of 3.8 cm.
6. Place the lubricated moulded with plungers in position in the load frame.
7. Apply the compressive load till the specimen is compacted to a height of 7.5 cm.
8. Eject the specimen from the constant volume mould.
9. Record the correct height, weight and diameter of the specimen.

Test procedure

1. Take two frictionless bearing plates of 75 mm diameter.
2. Place the specimen on the base plate of the load frame (sandwiched between the end plates).
3. Place a hardened steel ball on the bearing plate.
4. Adjust the center line of the specimen such that the proving ring and the steel ball are in the same line.
5. Fix a dial gauge to measure the vertical compression of the specimen.

6. Adjust the gear position on the load frame to give suitable vertical displacement.
7. Start applying the load and record the readings of the proving ring dial and compression dial for every 5 mm compression.
8. Continue loading till failure is complete.
9. Draw the sketch of the failure pattern in the specimen.

Sample details

Type UD/D: soil description

Specific gravity (G_s) 2.71

Bulk density :

Water content :

Degree of saturation % :

Diameter (D_o) of the sample ____ cm

Area of cross-section = ____ cm^2

Initial length (L_o) of the sample = 76 mm

Tab 15.1 Calculation sheet for Compressive stress determination

Elapsed time (minutes)	Compression dial reading (L) (mm)	Strain $L^* 100/L_o$ (%) (e)	Area A $A_o/(1-e)$ (cm^2)	Proving ring reading (Divns.)	Axial load (kg)	Compressive stress (kg/cm^2)

Interpretation of test Results

Unconfined compression strength of the soil = $q_u =$

Shear strength of the soil = $q_u/2 =$

Sensitivity = $(q_u \text{ for undisturbed sample}) / (q_u \text{ for remoulded sample}).$

General Remarks

Minimum three samples should be tested; correlation can be made between unconfined strength and field SPT value N. Up to 6% strain the readings may be taken at every min (30 sec).

Specimen Calculations

Experiment - 12

UNDRAINED TRIAXIAL TEST

Objective

To find the shear parameters of the soil by undrained triaxial test.

Need and scope

The standard consolidated undrained test is compression test, in which the soil specimen is first consolidated under all round pressure in the triaxial cell before failure is brought about by increasing the major principal stress.

It may be performing with or without measurement of pore pressure although for most applications the measurement of pore pressure is desirable.

Knowledge of equipment

A constant rate of strain compression machine of which the following is a brief description of one is in common use.

- a) A loading frame in which the load is applied by a yoke acting through an elastic dynamometer, more commonly called a proving ring which used to measure the load. The frame is operated at a constant rate by a geared screw jack. It is preferable for the machine to be motor driven, by a small electric motor.
- b) A hydraulic pressure apparatus including an air compressor and water reservoir in which air under pressure acting on the water raises it to the required pressure, together with the necessary control valves and pressure dials.

A tri axial cell is to take 3.8 cm dia and 7.6 cm long samples, in which the sample can be subjected to an all round hydrostatic pressure, together with a vertical compression load acting through a piston. The vertical load from the piston acts on a pressure cap. The cell is usually designed with a non-ferrous metal top and base connected by tension rods and with walls formed of Perspex.

Apparatus for preparation of the sample

- a) 3.8 cm (1.5 inch) internal diameter 12.5 cm (5 inches) long sample tubes.
- b) Rubber ring.
- c) An open ended cylindrical section former, 3.8 cm inside dia, fitted with a small rubber tube in its side.
- d) Stop clock.

- e) Moisture content test apparatus.
- f) A balance of 250 gm capacity and accurate to 0.01 gm.

Experimental Procedure

1. The sample is placed in the compression machine and a pressure plate is placed on the top. Care must be taken to prevent any part of the machine or cell from jogging the sample while it is being setup, for example, by knocking against this bottom of the loading piston. The probable strength of the sample is estimated and a suitable proving ring selected and fitted to the machine.
2. The cell must be properly set up and uniformly clamped down to prevent leakage of pressure during the test, making sure first that the sample is properly sealed with its end caps and rings (rubber) in position and that the sealing rings for the cell are also correctly placed.
3. When the sample is setup water is admitted and the cell is fitted under water escapes from the bleed valve, at the top, which is closed. If the sample is to be tested at zero lateral pressure water is not required.
4. The air pressure in the reservoir is then increased to raise the hydrostatic pressure in the required amount. The pressure gauge must be watched during the test and any necessary adjustments must be made to keep the pressure constant.
5. The handle wheel of the screw jack is rotated until the under side of the hemispherical seating of the proving ring, through which the loading is applied, just touches the cell piston.
6. The piston is then removed down by handle until it is just in touch with the pressure plate on the top of the sample, and the proving ring seating is again brought into contact for the beginning of the test.

Observations

1. The machine is set in motion (or if hand operated the hand wheel is turned at a constant rate) to give a rate of strain 2% per minute.
2. The strain dial gauge reading is then taken and the corresponding proving ring reading is taken the corresponding proving ring chart. The load applied is known.
3. The experiment is stopped at the strain dial gauge reading for 15% length of the sample or 15% strain.

Operator:

Sample No:

Date:

Job:

Location:

Size of specimen:

Length:

Proving ring constant:

Diameter:

Initial area L :

Initial Volume:

Strain dial gauge least count (const):

Tab 16.1 Tabulation sheet for tri axial test

Cell pressure kg/cm^2	Strain dial	Proving ring reading	Load on sample kg	Corrected area cm^2	Deviator stress
0.5	0				
	50				
	100				
	150				
	200				
	250				
	300				
	350				
	400				
	450				
0.5	0				
	50				
	100				
	150				
	200				

	250				
	300				
	350				
	400				
	450				
0.5	0				
	50				
	100				
	150				
	200				
	250				
	300				
	350				
	400				
	450				

Tab 16.2 Tabulation sheet for shear strength determination

Sample No.	Wet bulk density gm/cc	Cell pressure kg/cm ²	Compressive stress at failure	Strain at failure	Moisture content	Shear strength (kg/cm ²)	Angle of shearing resistance
1							
2							
3							

General Remarks

- a) It is assumed that the volume of the sample remains constant and that the area of the sample increases uniformly as the length decreases. The calculation of the stress is based on this new area at failure, by direct calculation, using the proving ring constant and the new area of the sample. By constructing a chart relating strain readings, from the proving ring, directly to the corresponding stress.
- b) The strain and corresponding stress is plotted with stress abscissa and curve is drawn. The maximum compressive stress at failure and the corresponding strain and cell pressure are found out.
- c) The stress results of the series of triaxial tests at increasing cell pressure are plotted on a mohr stress diagram. In this diagram a semicircle is plotted with normal stress as abscissa shear stress as ordinate.
- d) The condition of the failure of the sample is generally approximated to by a straight line drawn as a tangent to the circles, the equation of which is $\tau = C + \alpha \tan\phi$. The value of cohesion, C is read of the shear stress axis, where it is cut by the tangent to the mohr circles, and the angle of shearing resistance (ϕ) is angle between the tangent and a line parallel to the shear stress.

Specimen Calculations

Experiment - 13

DIRECT SHEAR TEST

Objective

Determine the shearing strength of the soil using the direct shear apparatus.

Need and scope

In many engineering problems such as design of foundation, retaining walls, slab bridges, pipes, sheet piling, the value of the angle of internal friction and cohesion of the soil involved are required for the design. Direct shear test is used to predict these parameters quickly. The laboratory reports cover the laboratory procedures for determining these values for cohesionless soils.

Apparatus

1. Direct shear box apparatus
2. Loading frame (motor attached)
3. Dial gauge
4. Proving ring
5. Tamper
6. Straight edge
7. Balance to weigh up to 200 mg
8. Aluminum container
9. Spatula

Knowledge of equipment

Strain controlled direct shear machine consists of shear box, soil container, loading unit, proving ring, dial gauge to measure shear deformation and volume changes. A two piece square shear box is one type of soil container used.

A proving ring is used to indicate the shear load taken by the soil initiated in the shearing plane.

Procedure

1. Check the inner dimension of the soil container.
2. Put the parts of the soil container together.
3. Calculate the volume of the container. Weigh the container.
4. Place the soil in smooth layers (approximately 10 mm thick). If a dense sample is desired tamp the soil.
5. Weigh the soil container, the difference of these two is the weight of the soil. Calculate the density of the soil.
6. Make the surface of the soil plane.

7. Put the upper grating on stone and loading block on top of soil.
8. Measure the thickness of soil specimen.
9. Apply the desired normal load.
10. Remove the shear pin.
11. Attach the dial gauge which measures the change of volume.
12. Record the initial reading of the dial gauge and calibration values.
13. Before proceeding to test check all adjustments to see that there is no connection between two parts except sand/soil.
14. Start the motor. Take the reading of the shear force and record the reading.
15. Take volume change readings till failure.
16. Add 5 kg normal stress 0.5 kg/cm^2 and continue the experiment till failure
17. Record carefully all the readings. Set the dial gauges zero, before starting the experiment

Data Calculation Sheet For Direct Shear Test

Normal stress 0.5 kg/cm^2 L.C=_____ P.R.C =_____

Normal stress 1.0 kg/cm^2 L.C=_____ P.R.C =_____

Normal stress 1.5 kg/cm^2 L.C=_____ P.R.C =_____

The Below table is used for all the above normal stress of $0.5, 1.0, 1.5 \text{ kg/cm}^2$ separately with respect to their L.C and P.R.

Tab. 14.1 Observation and Calculation sheet for shear stress determination

Horizontal Gauge Reading	Vertical Dial gauge Reading	Proving ring Reading	Hori. Dial gauge Reading Initial reading div. gauge	Shear deformation Col.(4) x Leastcount of dial	Vertical gauge reading Initial Reading	Vertical deformation = div.in col.6 x L.C of dial gauge	Proving reading Initial reading	Shear stress = $\frac{\text{div.col.(8)} \times \text{proving ring constant Area of the specimen}(\text{kg/cm}^2)}{\text{div.col.(8)}}$
1	2	3	4	5	6	7	8	9
0								
25								
50								
75								
100								
125								
150								
175								
200								
250								
300								
400								
500								
600								
700								
800								
900								

Observations

Proving Ring constant _____ least count of the dial _____
Calibration factor _____
Leverage factor _____
Dimensions of shear box 60 x 60 mm
Empty weight of shear box _____
Least count of dial gauge _____
Volume change _____

Tab 14.2 Observation sheet for shear stress

S.No	Normal load (kg)	Normal stress(kg/cm ²) load x leverage/Area	Normal stress(kg/cm ²) load x leverage/Area	Shear stress Proving Ring reading x calibration / Area of container
1				
2				
3				

General Remarks

1. In the shear box test, the specimen is not failing along its weakest plane but along a predetermined or induced failure plane i.e. horizontal plane separating the two halves of the shear box. This is the main draw back of this test. Moreover, during loading, the state of stress cannot be evaluated. It can be evaluated only at failure condition i.e. Mohr's circle can be drawn at the failure condition only. Also failure is progressive.
3. Direct shear test is simple and faster to operate. As thinner specimens are used in shear box, they facilitate drainage of pore water from a saturated sample in less time. This test is also useful to study friction between two materials. One material in lower half of box and another material in the upper half of box.
3. The angle of shearing resistance of sands depends on state of compaction, coarseness of grains, particle shape and roughness of grain surface and grading. It varies between 28°(uniformly graded sands with round grains in very loose state) to 46°(well graded sand with angular grains in dense state).

4. The volume change in sandy soil is a complex phenomenon depending on gradation, particle shape, state and type of packing, orientation of principal planes, principal stress ratio, stress history, magnitude of minor principal stress, type of apparatus, test procedure, method of preparing specimen etc. In general loose sands expand and dense sands contract in volume on shearing. There is a void ratio at which either expansion contraction in volume takes place. This void ratio is called critical void ratio. Expansion or contraction can be inferred from the movement of vertical dial gauge during shearing.
5. The friction between sand particles is due to sliding and rolling friction and interlocking action.

The ultimate values of shear parameter for both loose sand and dense sand approximately attain the same value so, if angle of friction value is calculated at ultimate stage, slight disturbance in density during sampling and preparation of test specimens will not have much effect.

Specimen Calculations

Experiment - 14

VANE SHEAR TEST

Objective

Determine the shear strength of a given soil specimen.

Need and scope

The structural strength of soil is basically a problem of shear strength. Vane shear test is a useful method of measuring the shear strength of clay. It is a cheaper and quicker method. The test can also be conducted in the laboratory. The laboratory vane shear test for the measurement of shear strength of cohesive soils is useful for soils of low shear strength (less than 0.3 kg/cm^2) for which triaxial or unconfined tests cannot be performed. The test gives the undrained strength of the soil. The undisturbed and remoulded strength obtained are useful for evaluating the sensitivity of soil.

Equipment

1. Vane shear apparatus
2. Specimen
3. Specimen container
4. Calipers

Experimental procedure

1. Prepare two or three specimens of the soil sample of dimensions of at least 37.5 mm diameter and 75 mm length in specimen. (L/D ratio 2 or 3).
2. Mount the specimen container with the specimen on the base of the vane shear apparatus. If the specimen container is closed at one end, it should be provided with a hole of about 1 mm diameter at the bottom.
3. Gently lower the shear vanes into the specimen to their full length without disturbing the soil specimen. The top of the vanes should be at least 10 mm below the top of the specimen. Note the readings of the angle of twist.
4. Rotate the vanes at a uniform rate say $0.1^\circ/\text{s}$ by suitable operating the torque application handle until the specimen fails.
5. Note the final reading of the angle of twist.
6. Find the value of blade height in cm.
7. Find the value of blade width in cm.

Calculations & Observations

$$\text{Shear strength, } S = \frac{T}{\pi(D^2 H / 2 + D^3)}$$

Where S = shear strength of soil in kg/cm²

T = torque in cm kg

D = overall diameter of vane in cm

T = spring constant / 180° x difference in degrees.

Tab 13.1 Observation sheet for shear strength determination

S.No	Initial Reading (Deg)	Final Reading (Deg.)	Difference (Deg.)	T=Spring Constant* Difference/180 Kg-cm	$G = \frac{1}{\pi} \left(\frac{D^2 H}{2} + \frac{D^3}{6} \right)$	S=TxG Kg/cm ²	Average 'S' Kg/cm ²	Spring Constant Kg-cm

General Remarks

This test is useful when the soil is soft and its water content is nearer to liquid limit.

Specimen Calculations

Experiment - 15

STANDARD PENETRATION TEST

Objective

To perform standard penetration test and the penetration resistance (N-value) along the depth at a given site.

Apparatus

- Tripod (to give a clear height of about 4 m; one of the legs of the tripod should have ladder to facilitate a person to reach tripod head.)
- Tripod head with hook
- Pulley
- ¹Guide pipe assembly
- Standard split spoon sampler
- A drill rod for extending the test to deeper depths
- Heavy duty post hole auger (100 mm to 150 mm diameter)
- Heavy duty helical auger
- Heavy duty auger extension rods
- Sand bailer
- Rope (about 15 m long & strong enough to lift 63.5 kg load repeatedly)
- A light duty rope to operate sand bailer
- Chain pulley block
- Casing pipes
- Casing couplings
- Casing clamps
- Measuring tapes
- A straight edge (50 cm)
- Tool box

Procedure

1. Identify the location of testing in the field
2. Erect the tripod such that the top of the tripod head is centrally located over the testing spot. This can be reasonably ensured by passing a rope over the pulley connected to the tripod head and making the free end of the rope to come down and adjusting the tripod legs such that the rope end is at the testing spot. While erecting and adjusting the tripod legs, care should be taken to see that the load is uniformly distributed over the three legs. This can be achieved by ensuring the lines joining the tips of the tripod legs on the ground forms an equilateral triangle. Further, it should be ensured that the three legs of the tripod are firmly supported on the ground (i.e. the soil below the legs should not be loose and they should not be supported on a sloping rock surface or on a small boulder which may tilt during testing.)
3. Advance the bore hole, at the test location, using the auger. To start with advance the bore hole for a depth of 0.5 m and clear the loose soil from the bore hole.

4. Clean the split spoon sampler and apply a thin film of oil to the inside face of the sampler. Connect an A-drill extension rod to the split spoon sampler.
5. Slip the 63.6 kg weight on to the guide pipe assembly and connect the guide pipe assembly to the other end of the A-drill rod.
6. The chain connected to the driving weight is tied to the rope passing over the pulley at the tripod head. The other end of the rope is pulled down manually or with help of mechanical winch. By pulling the rope down, the drive weight, guide pipe assembly, A-drill rod and the split spoon sampler will get vertically erected.
7. A person should hold the guide pipe assembly split spoon sampler to be vertical with the falling weight lowered to the bottom of the guide assembly.
8. Now place a straight edge across the bore touching the A-drill rod. Mark the straight edge level all round the A-drill rod with the help of a chalk or any other marker. From this mark, measure up along the A-drill rod and mark 15 cm, 30 cm and 45 cm above the straight edge level. Lift the driving weight to reach the top of the guide pipe assembly travel and allow it to fall freely. The fall of driving weight will transfer the impact load to the split spoon sampler, which drive the split spoon sampler into the ground. Again lift the drive weight to the top of travel and allow it to fall freely under its own weight from a height of 75 cm. as the number of blows are applied, the split spoon sampler will penetrate into the ground and the first mark (15 cm mark) on the drill rod approaches the straight edge.
9. Count the number of blows required for the first 15 cm, second 15 cm and the third 15 cm mark to cross down the straight edge.
10. The penetration of the first 15 cm is considered as the seating drive and the number of blows required for this penetration is noted but not accounted in computing penetration resistance value. The total number of blows required for the penetration of the split spoon sampler by 2nd and 3rd 15 cm is recorded as the penetration resistance or N-value.
11. After the completion of the split spoon sampler by 45 cm, pull out the whole assembly. Detach the split sampler from A-drill rod and open it out. Collect the soil sample from the split spoon sampler into a sampling bag. Store the sampling bag safely with an identification tag for laboratory investigation.
12. Advance the bore hole by another 1 m or till a change of soil strata which ever is early.
13. The test is repeated with advancement of bore hole till the required depth of exploration is reached or till a refusal condition is encountered. Refusal condition is said to exist if the number of blows required for the last 30 cm of penetration is more than 100.
14. The test will be repeated in number of bore holes covering the site depending on the building area, importance of the structure and the variation of the soil properties across the site.
15. The SPT values are presented either in the form of a table or in the form of bore log data.

Result: The SPT Value N =